

Insights from dynamical physics-based models of contaminant transport in river flow

ITT 19: Wessex Water Challenges 2 and 3

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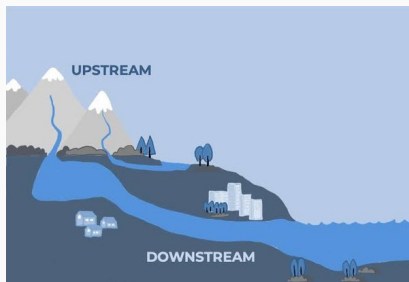
University of Bath

Motivation and challenges

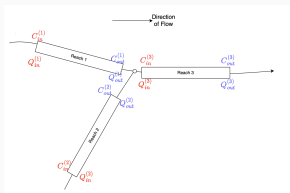
Wessex water is interested in understanding:

- **Effects/predictions of velocity:** velocity is a key ingredient in predictions; how can it be better estimated?
- **Source apportionment:** how can we attribute original upstream sources of contaminants from downstream measurements.
- Effects of UV exposure on transport and decay

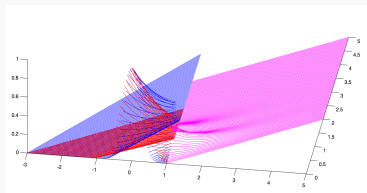
Our approach: develop dynamic mathematical models



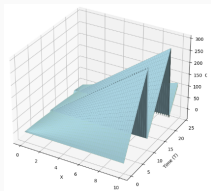
Overview of approaches



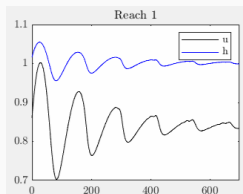
(a) Simplified network model of contaminants



(b) Dynamic network model of contaminants



(c) Exploring functions influencing the concentration profile

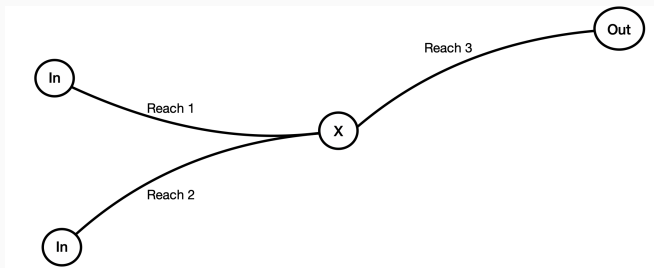


(d) Varying velocity and flow rates

Toy setup: the 3-reach geometry

We consider a simplified geometry divided into three river sections (“reaches”).

- Contaminants injected (or measured) in Reach 1 and Reach 2 (“in”)
- Key downstream measurement at Reach 3 (“out”)



1D physics based model for bacteria concentration, $c_i(x_i, t)$, and velocity, $u_i(x_i, t)$, $i = 1, 2, 3$.

- **Concentration:** advection-diffusion-reaction equation in each reach:

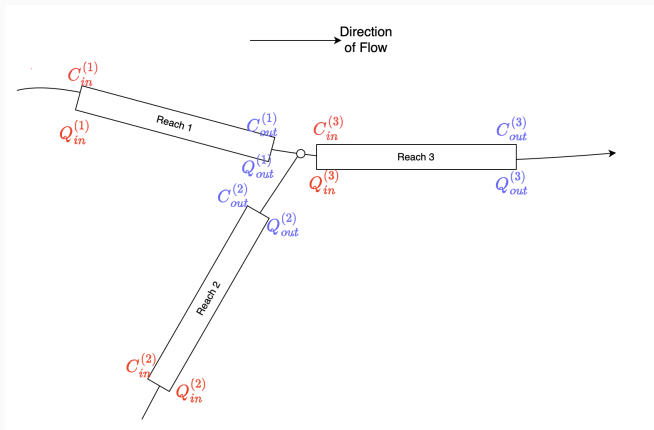
$$\frac{\partial c_i}{\partial t} + \underbrace{u_i \frac{\partial c_i}{\partial x_i}}_{\text{advection}} = k \underbrace{\frac{\partial^2 c_i}{\partial x_i^2}}_{\text{diffusion}} + [\text{source terms}] + [\text{decay terms}],$$

where x_i is the arc length coordinate along the river.

- **Velocities:** determined using the St. Venant shallow water equations describing river flow:

$$\underbrace{\frac{\partial u_i}{\partial t} + u_i \frac{\partial u_i}{\partial x_i}}_{\text{acceleration}} + \underbrace{g \frac{\partial h_i}{\partial x_i}}_{\text{pressure}} = g \underbrace{\frac{S_f}{h_i}}_{\text{friction}} - \underbrace{g S_0}_{\text{gravity}},$$
$$\frac{\partial h_i}{\partial t} + \frac{\partial (h_i u_i)}{\partial x_i} = [\text{source terms}].$$

Developing a simplified graph-based model I



River network with 3 segments.

Red: inputs of the reach models. Blue: outputs of the reaches.

Developing a simplified graph-based model II

In each reach model, we assume

- No internal mass change within the reach
- Reach flows are constant and governed by a law:

$$\mathbf{F}(Q_1, Q_2, \dots, Q_N) = 0 \implies Q_{\text{in}} = Q_{\text{out}}.$$

We assume the flows are known in the procedure; either approximated via simple mass balance or simulated via PDE models

- Contaminants obey a mass balance law:

$$\mathbf{G}(\mathbf{C}; \mathbf{Q}) = 0 \implies C_{\text{in}}^{(3)} = \frac{Q_1 C_{\text{out}}^{(1)} + Q_2 C_{\text{out}}^{(2)}}{Q_1 + Q_2}.$$

- Velocities assumed known via empirical law: $v = aQ^b$, after which travel time in each reach determined via $\tau = \text{length}/\text{velocity}$.
- Once travel time is known, contaminant concentration can be predicted.

- Let us assume the decay rate is constant and contaminants obey

$$\frac{\partial C_i}{\partial t} = -k_i C_i.$$

$\implies C_i = C_{i0} e^{-k_i t}$ for each reach.

- Then the output concentration at reach 3 is

$$C_{out}^{(3)} = \Gamma_1 e^{-\Lambda_1} + \Gamma_2 e^{-\Lambda_2},$$

where $\Gamma_i(C_{in}^{(i)}, Q_i)$ and $\Lambda_i(k_i, L_i, Q_i)$ are completely known.

- For example, concentration behaviour is **strongly dominated** by the Lambda-functions:

$$\Lambda_1 = k_1 \frac{L_1}{Q_1} + k_3 \frac{L_3}{Q_1 + Q_2}$$

i.e. decay dictated by reach properties.

Simplicity of such approximate decay laws facilitate intuition of processes.

- Consider the effect of erroneous prediction of a flow measurement, $Q_1 \mapsto \bar{Q}_1 + \delta Q$. It can be verified this modifies decay prediction:

$$\log(C_{out}^{(3)}) \propto (-\bar{\Lambda} - \mathcal{P}\delta Q)t$$

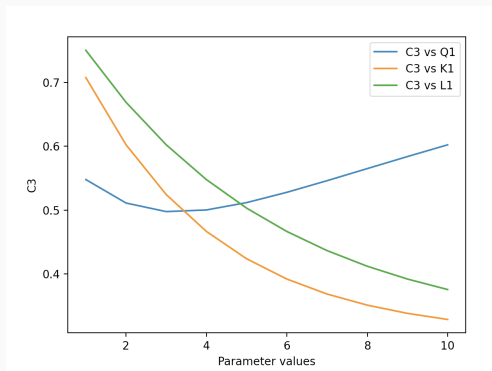


Figure 2: Parametric sensitivity analysis

Now including transport/velocity, the 1D advection-diffusion-reaction equations can be solved in closed form.

- Concentration $C(x, t)$ in each reach follows

$$\frac{\partial C^{(j)}}{\partial t} + u(Q_j) \frac{\partial C^{(j)}}{\partial x} = -k(t)C^{(j)}$$

I.C. $C^{(j)}(x, 0) = C_{\text{in}}^{(j)}(x), \quad j = 1, 2$

$$C^{(3)}(x, 0) = 0$$

- Concentration at the junction

$$C_i^{(3)}(t) = \begin{cases} 0 & t < t_1^* \\ \frac{Q_1 C_{\text{out}}^{(1)}(L_1, t_1^*)}{Q_1 + Q_2} & t_1^* \leq t < t_2^* \\ \frac{Q_1 C_{\text{out}}^{(1)}(L_1, t_1^*) + Q_2 C_{\text{out}}^{(2)}(L_2, t_2^*)}{Q_1 + Q_2} & t \geq t_2^* \end{cases}$$

- In each reach, solution is combination of wave and decay:

$$C(x, t) = \underbrace{C_{\text{in}}(x - ut)}_{\text{wave phenom}} \underbrace{e^{-K(x, t)}}_{\text{decay}},$$

$$K(x, t) = \int_{t-x/u}^t k(t') dt'$$

where all three solutions must be matched at the junction.

- Example diurnal effects:
 $k(t) = A \cos(\pi t/12) + B$

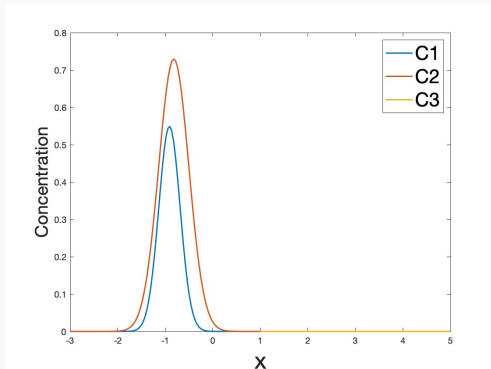
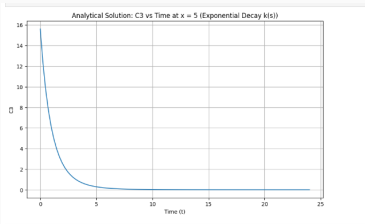


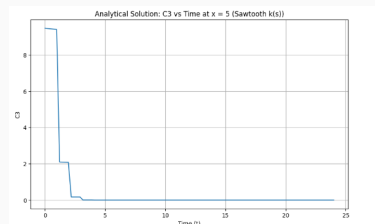
Figure 3: I.C. for first two reach models

Conclusion: highly-efficient formulae for concentration decay and transport

Exploring suitable functions for $k(t')$



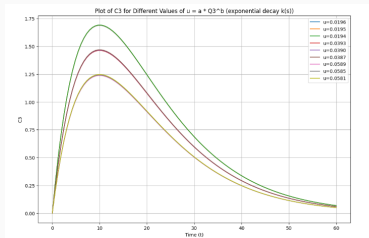
(a) Exponential decay for smooth behaviour



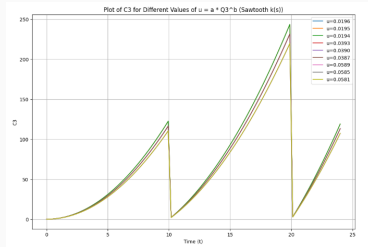
(b) Sawtooth function for periodic decay

- Exponential Decay: $k(t') = e^{-at'}$ - continuous decay patterns.
- Sawtooth function: $k(t') = \text{mod}(s, \text{period})$ - periodic/cyclic decay processes.

Influence of u and $k(t)$ in the analytical solution



(a) Exponential decay(u values)

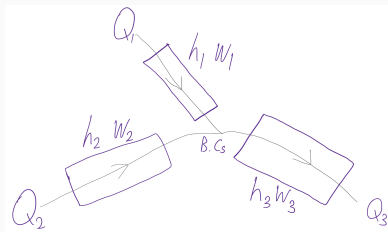


(b) Sawtooth function(u values)

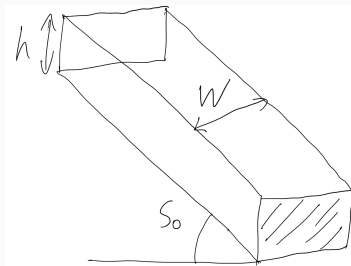
- High Q (flux) - Steeper slopes and faster rate of decay
- Low Q (flux) - gentler slopes and lower decay rates over time

Approach 2: PDE model (3 rivers)

- **Challenge:** Can we use a PDE model to develop dynamic (spatial-temporal-varying) river velocities?



(a) 3 rivers catchment



(b) Cuboid shape for a river

- Assume cuboid river. Width (w), energy slope (S_0) and initial heights are known

Find u_i and h_i from the St. Venant (shallow river) conservation of momentum and conservation of mass equations ($i = 1, 2, 3$):

$$\underbrace{\frac{\partial u_i}{\partial t} + u_i \frac{\partial u_i}{\partial x_i}}_{\text{acceleration}} + \underbrace{g \frac{\partial h_i}{\partial x_i}}_{\text{pressure}} = \underbrace{g \frac{S_f}{h_i}}_{\text{friction}} - \underbrace{g S_0}_{\text{gravity}},$$

$$\frac{\partial h_i}{\partial t} + \frac{\partial (h_i u_i)}{\partial x_i} = [\text{source terms}],$$

where S_f is found using Manning's Law,

$$S_f = \frac{u_i^2 n^2 (2h_i + w_i)^{4/3}}{(h_i w_i)^{4/3}}.$$

Equation for the flow, Q_i , linking u_i and h_i :

$$Q_i = w_i h_i u_i.$$

- For reaches 1 and 2, we specify Q at the upstream end ($x = 0$).
- Modelled $Q_i = A_i \sin(\omega_i t)$ to reflect the diurnal flow variation.
- At the river junction, require continuity of momentum/mass:

$$Q_1 + Q_2 = Q_3,$$

$$h_1 w_1 + h_2 w_2 = h_3 w_3.$$

- Imposed simplified boundary conditions on reach 3:

$$Q_3 = \frac{Q_1 + Q_2}{2}, \quad h_3 = \frac{h_1 + h_2}{2}.$$

Gif Animation!

Conclusions

- Simplified network models can provide **powerful machinery** for intuition and approximation.
- Sensitivity analysis and optimisation is possible within simplified physics-based frameworks.
- Physics-based PDE models for river flows can provide detailed predictions of transport phenomena.

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Next steps

- Develop framework for **data-driven simulations**: *Can we couple uncertainties with physics-based modelling?*
- **Upscaling**: Incorporate realistic river geometries, source effects, storm overflows.
- Can we augment a network model with PDE behaviours?
- Combine varying velocity field with simplified concentration models.
- Source apportionment: Inverse problem for unknown concentrations.



"That's all Folks!"

